

The Formation and Stabilization of Aerobic Granular Sludge in a Sequencing Batch Airlift Reactor for Treating Tapioca-Processing Wastewater

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Abstract

This study evaluated performance and granule features of a sequencing batch airlift reactor (SBAR) to treat tapioca-processing wastewater. The effect of organic loading rate (OLR) on the stabilization of aerobic granules was also investigated by increasing the OLR gradually from 2.5 kgCOD/m³.day to 10 kgCOD/m³.day. The results indicated that clear boundary granules were formed after a two-week cultivation period. The aerobic granules' average diameter increased according to rise in OLR and got a stable value of 2.5 mm at an OLR of 7.5 kgCOD/m³.day. The mature granules consisted of a dark core (anoxic) and yellow sludge surroundings (aerobic). The higher OLR led to forming granules of diameter 3–4 mm which were then broken due to substrate diffusion limitation. Aerobic granules could successfully treat organic substances, NH₄⁺-N, and phosphorus with high removal efficiencies of 93.9–96.3%, 79.7–82.6%, and 80–95%, respectively. We observed that the aerobic granular sludge has good settling ability with a sludge volume index (SVI) lower than 50 mL/g, and is able to withstand high OLR. The experimental findings created a new prospect for granulation and employment of aerobic granules to treat industrial wastewater.

Keywords: aerobic granules, granulation, tapioca processing wastewater, organic substance removal, sequencing batch airlift reactor (SBAR)

Introduction

During the 1990s conventional aerobic biological processes for wastewater treatment – namely activated

sludge, biofilter, and rotating biological reactor – were studied and applied in domestic and industrial wastewater treatment contexts. However, this technology has several disadvantages, such as low organic loading rates (0.5–2 kgCOD/m³.day), shock loading, and high excess biomass concentration. In recent years the technological improvement studies have focused significantly on aerobic

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granular sludge, which has advantages that overcome the drawbacks of activated sludge such as settling velocity greater than 10 m/h [1], SVI less than 35 mL/g [2], and high organic loading rates of 15 kg COD/m³.day [3].

Aerobic granular sludge is defined as aggregate of microbial origin that does not coagulate under reduced hydrodynamic shear, and which settles significantly faster than activated sludge flocs. It is developed under aerobic conditions and mainly used for, firstly, the aerobic degradation of organics; and secondly, removing nitrogen under aerobic and anoxic conditions [4]. The high biomass retention property of aerobic granular sludge enables the organic loading rate of a reactor to increase. A long sludge retention time is also especially beneficial to nitrifying bacteria. Aerobic and anoxic zones are present inside the granules, which can simultaneously conduct the nitrification and denitrification processes, and therefore nitrogen pollution in wastewater can be removed. Furthermore, aerobic granular sludge can be used to treat various wastewaters and is able to tolerate shock loading due to the unique granular structure and high biomass concentration in the reactor [5]. Aerobic granular sludge was first studied in an aerobic upflow sludge blanket reactor for municipal sewage treatment [6]. Other reactors with strong mixing ability were also employed for forming aerobic granular sludge, including a sequencing batch reactor (SBR) [7-9] and a sequencing batch airlift reactor (SBAR) [10-13]. The elliptical-shaped airflow improved the mixing speed in SBAR, inducing the granules become more compact and steady.

Most of the aerobic granule formation studies were carried out on synthetic wastewater consisting of glucose, sucrose, acetate, and ethanol [3, 9, 14-16], and some attempts have applied aerobic granular sludge in practical wastewater treatment for treating pollutants in domestic wastewater [17], municipal wastewater [7], and industrial wastewater treatments [2, 8, 18]. However, the aerobic granule formation in these cases remains unclear and the effect of aerobic granules on reducing organics in lots of industrial wastewater is lacking. Furthermore, there have been no reports regarding the use of aerobic granules on tapioca-processing wastewater – one of the most prevalent sources of pollution in Vietnam [19]. Therefore, this study aimed to investigate the formation and stabilization of aerobic granules from the acclimation stage to mature aerobic granules on tapioca-processing wastewater treatment. The performance of SBAR on treatment of the wastewater was measured in terms of removal efficiency of COD, nitrogen, and phosphorus.

Experimental Procedures

Tapioca-Processing Wastewater and Seed Sludge Condition

The wastewater sample used in this study was collected from the equalization tank of a tapioca processing factory in Tay Ninh Province, Vietnam. Samples were collected

Table 1. Characterization of raw tapioca-processing wastewater.

Parameters	Unit	Values	Vietnamese standard of industrial wastewater discharge (Column B QCVN 40:2011/BTNMT)
pH	-	3.9-4.5	5.5-9.0
COD	mg/L	4,800-16,000	150
BOD ₅	mg/L	2,500-11,550	50
CN ⁻	mg/L	2-75	0.1
SS	mg/L	350-1,000	100
NH ₄ ⁺ -N	mg/L	95-182	10
Total-N	mg/L	145-470	40
PO ₄ ³⁻ -P	mg/L	127-432	6

and stored in 20-L containers and kept at 4°C from February to September 2015. The composition of tapioca processing wastewater is given in Table 1.

To enhance the development of microorganisms, the influent wastewater was supplemented by nutrient ingredients (N, P) and micronutrients at doses according to Wang et al. [20]. The composition of tapioca-processing wastewater is given in Table 1. The biogas sludge was used for microbial cultivation with a mixed liquor suspended solid (MLSS) concentration of 3,627 mg/L (VSS/SS ratio of 50%). The pH value of influent wastewater was maintained at about 6.8-7.2 using NaHCO₃ 1 M.

Reactor Set-up

Experiments were done in an open, cylindrical column-type SBAR, with a working volume of 3 L (Fig. 1). The reactor was made from 2 mm thick acrylic material,

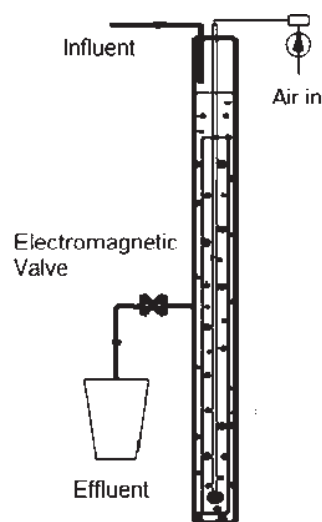


Fig. 1. Schematic diagram of sequencing batch airlift reactor (SBAR) setup.

including two columns. The external column is 1,000 mm in height and has a diameter of 70 mm. The internal column with a diameter of 40 mm and 700 mm height was positioned 15 mm from the bottom of the external column.

Air was introduced through a pumice stone located at the bottom of the reactor, and airflow rate of 5 L/min was applied. The reactor has an automatic control system with an electromagnetic valve 400 mm above the bottom of the external column to drain the water (50% of the water volume in the reactor was withdrawn).

Experimental Procedure

The reactor was operated in sequencing batch mode, feeding and withdrawing automatically. Each three-hour cycle consisted of four steps: 5 min of influent filling, 158-170 min of aeration, 3-15 min of settling, and 2 min of effluent withdrawal. Time for influent filling and effluent withdrawal remained consistent during the experiments while settling time was decreased step by step from 15 min to 3 min in the first two weeks to enhance the washing of poor-settling ability sludge and retained granules in which the settling velocity was around 8 m/h. The reactor was operated at the step-wise increased OLR of 2.5, 3.2, 5.0, 7.5, and 10 kgCOD/m³.day as presented in Table 2.

The experiments were executed in the laboratory at an ambient temperature ranging from 28 to 32°C.

Analytical Methods

Wastewater samples taken from the reactor were analysed for dissolved oxygen (DO), pH, COD, total nitrogen (TN), NH₄⁺-N, NO₃⁻-N, NO₂⁻-N, total phosphorus (TP), and alkalinity. The sludge samples were analyzed for mixed liquor suspended solids (MLSS), mixed liquor volatile suspended solids (MLVSS), and sludge volume index (SVI), as well as diameter, surface structure, and inner microbial organization of the granules. DO and pH were determined by the electrode probe using a portable Hach pH/DO meter (HQ40d model). COD, N, P, SS, and SVI were examined according to the standard methods [21].

Granule diameter was characterized using an Olympus BX 51 microscope with attached DP 71 camera and image analysis software. The sludge structure and inner microbial organization were determined according to the crystal violet method [22].

All experiments were performed in triplicate and the average values were reported.

Table 2. Organic loading rate (OLR) over the entire experimental period.

Week	1-3	4-8	9-14	15-16	17-18
OLR, kgCOD/m ³ .day	2.5	3.2	5.0	7.5	10

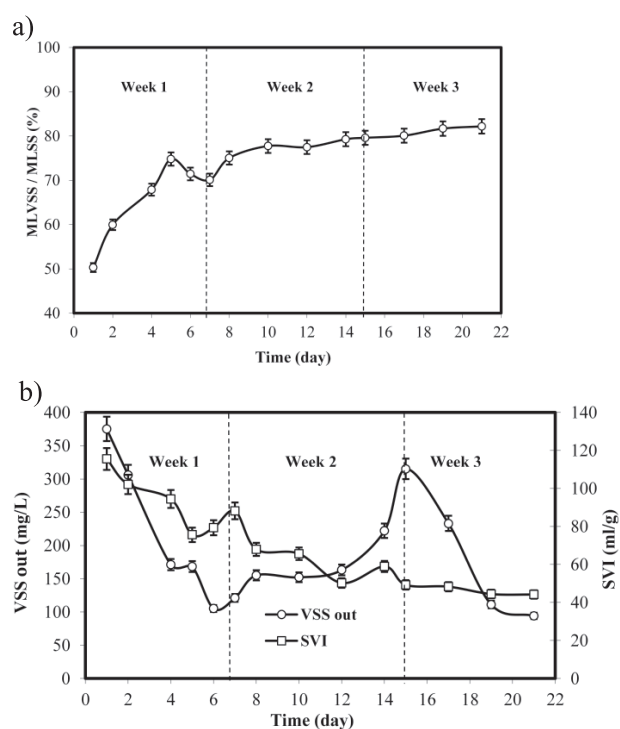


Fig. 2. Change of: a) MLVSS/MLSS ratio and b) VSS concentration in effluent and SVI in reactor at OLR of 2.5 kgCOD/m³.day.

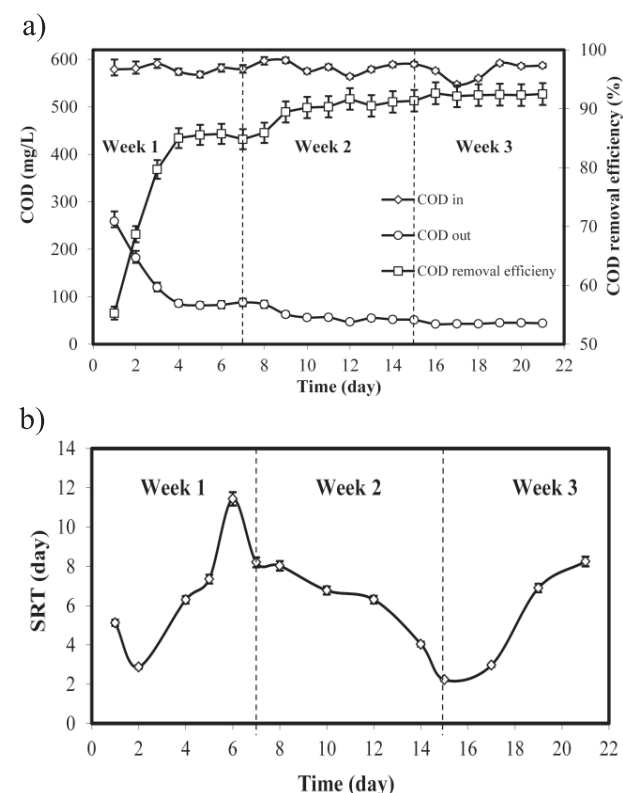


Fig. 3. Change of: a) COD and b) sludge retention time at OLR of 2.5 kg COD/m³.day.

Results and Discussion

Formation and Stabilization of Aerobic Granules

After one week of acclimation at the organic loading rate (OLR) of 2.5 kgCOD/m³.day, anaerobic biogas sludge was transformed completely into aerobic sludge, as indicated by the color of the sludge turning from black

to dark brown. The rate of MLVSS/MLSS increased from 50% to 75% (Fig. 2a) and the sludge volume index (SVI) decreased from 115 mL/g to 90 mL/g (Fig. 2b), showing that the bioactivity and sludge settleability was better. At that time, settling time decreased gradually to 4 min at the end of the 2nd week to remove the bad settleability flocs and hasten granulation. The flocs aggregated and became heavier, leading to decreases in SVI to 50 mL/g. As the small flocs were washed out, biomass content in

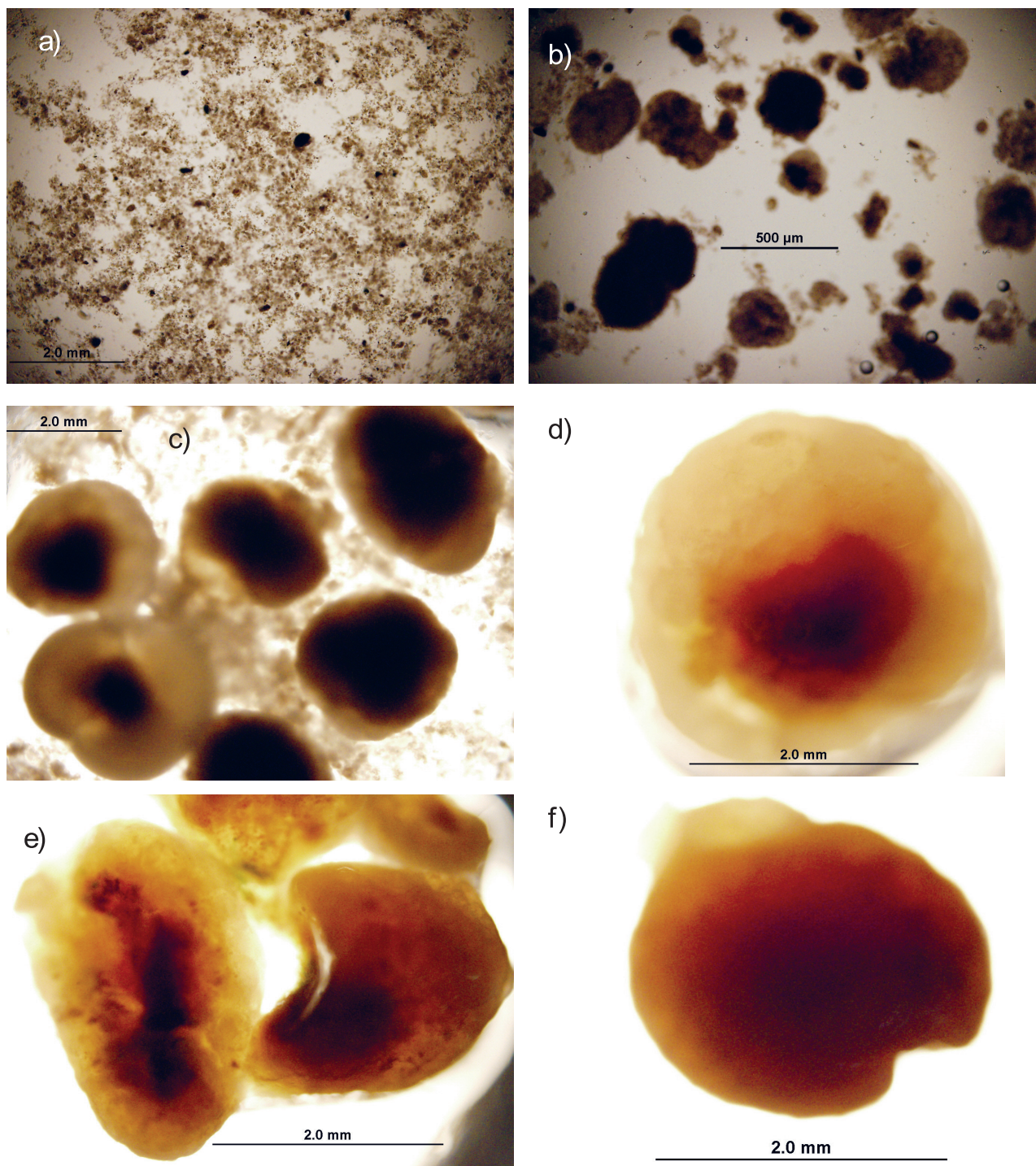


Fig. 4. Granules in different weeks: a) seed sludge, b) small granules in the third week, c) mature granules after week 10, d) mature granule at week 16, e) broken granules, and f) granular core.

the reactor declined while the concentration of volatile suspended solid (VSS) of effluent increased from 152 mg/L up to 325 mg/L on the day 15 (Fig. 2b) and short sludge age varied between 2-6 days (Fig. 3b). From the third week forward, the settling time was maintained at a constant value of 3 min. At this time, color of the sludge switched from dark brown to light brown, and sludge flocs had a tendency to segregate. The small granules appeared to be streak-shaped, which had an average diameter of 200 μm (Fig. 4b). As the granules were formed; the rate of MLVSS/MLSS also increased rapidly and reached more than 80% (Fig. 2a), and SVI was lower than 50 mL/g due to the increase in the dimension of granules.

Obviously, operating the reactor with the step-wise decreased settling time, the period time for small granules appearing in this experiment was similar to the results of other studies, often after an operational 2-3 weeks [23-25]. However, physical characteristic of granules were different due to various operational conditions. These variations in characteristics of granules could be recorded in the reports of Kong et al. [23] and Liu and Tay [25]. While at similar operating conditions with an OLR of 3 kg COD/m³.day and hydraulic retention time (HRT) of 4 h, Kong et al. [23] cultivated small aerobic granules after two weeks in the acetate-based synthetic wastewater. The granules reached an average diameter of about 200 μm , and SVI 70 mL/g. Liu and Tay [25] got the aerobic granules with SVI increased from 108 mg/L to around 170 mL/g. An explanation for these variations could be the high OLR of 8 kgCOD/m³.day, which could promote granulation with larger dimensions but let structure loose, which may lead to higher VSI in the first two weeks. Therefore, acclimatizing the operation to the moderate OLR-induced the formation of granules had a more compact structure [20, 24].

To investigate the effect of the OLR on stability of aerobic granules, the OLR increased gradually from 3.2 kg COD/m³.day up to 10 kg COD/m³.day. Along with an increase in OLR, granules appeared and increased in both diameter and density (Fig. 5). The structure of granules reached stability at a diameter of 2 mm after being operational for 10 weeks (Fig. 4c) and maintained stability until the week 16 at a diameter of 2.5 mm, when the OLR rose to 7.5 kg COD/m³.day (Fig. 4d). The mature granules had a compact structure, and round and smooth surface included the dark core with the presence of dead cells and anaerobic bacteria, and surroundings were a yellow aerobic layer with the presence of nitrification and heterotrophic bacteria. Most other research has produced similar results in the structure of aerobic granules, which included the interior and surrounding anaerobic layers [26, 27]. Up to operational condition, the mature granules had an average diameter in the range of 1.5-3mm and SVI of around 50 mL/g. These results correspond with average granule diameter of 1.8 mm and SVI of 50 mg/L after 16 weeks of operation [23]. This implies that tapioca-processing wastewater was in good condition for granule formation.

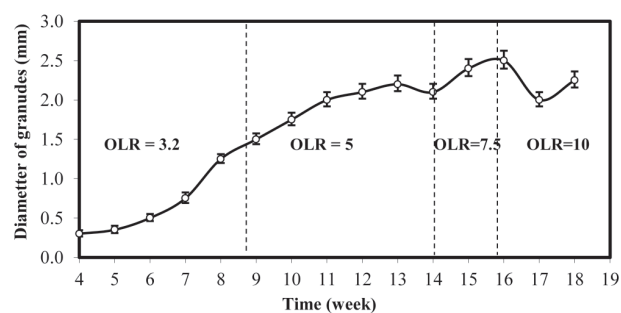


Fig. 5. Change in granule diameter at different OLRs (kgCOD/m³.day).

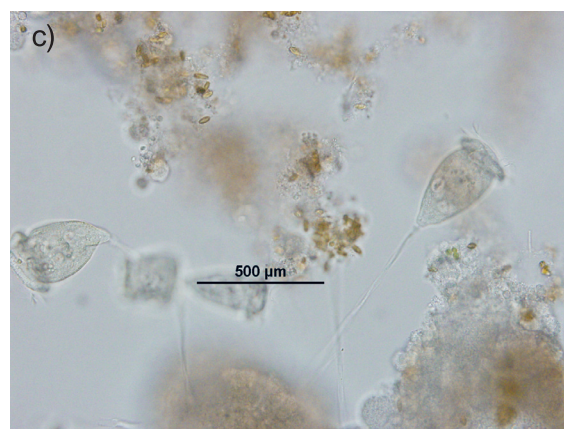
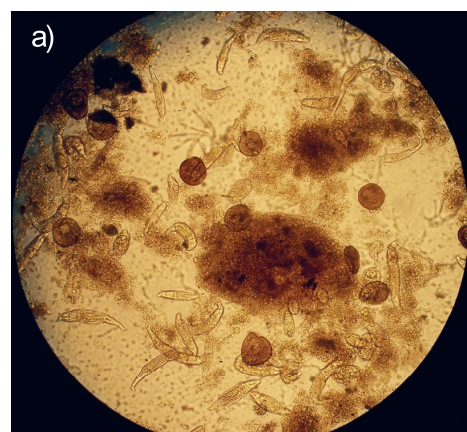


Fig. 6. Protozoa in sludge: a) crawling ciliates, b) free-swimming ciliates, and c) stalked ciliates.

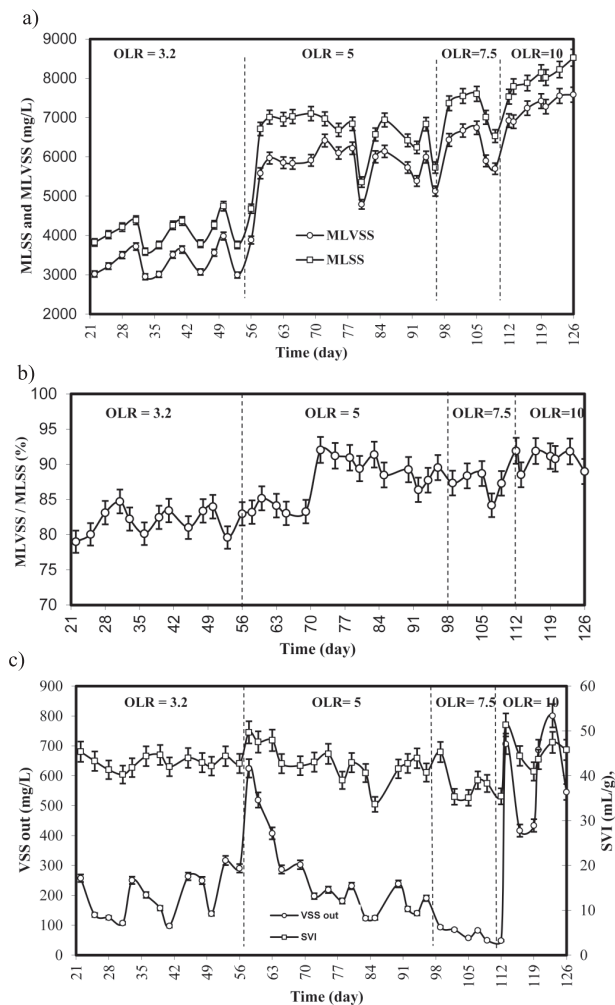


Fig. 7. Variation in parameters at different OLRs: a) MLSS, MLVSS concentration, b) MLVSS/MLSS ratio, and c) VSS_{out} and SVI.

However, when the OLR rose to 10 kg COD/m³.day, the diameter increased to 3-4 mm. This was difficult for the substances diffusing into the granules' core, which caused the granules to crack (Fig. 4e). As a result, the outer layer was broken while the black core remained (Fig. 4f). Subsequently, the broken granules recovered quickly, aggregated, and increased the biomass. Since the large granules had a thick anaerobic layer induced to anaerobic fermentation that released biogas, organic acid could disintegrate these granules [26]. Obviously, aerobic granules were stable at the maximum OLR of 7.5 kg COD/m³.day in the operational condition of this experiment. At this OLR, the granules had compact structure and good settling ability indicated by the low SVI of around 35-40 mL/g and VSS concentration in the effluent lower than 100 mg/L (Fig. 7c). The trends of granules characteristic during OLR variation were also described by Liu and Tay [25], who successfully cultivated aerobic granules at the high OLR of 12 kg COD/m³.day and maintained the stability of granules (SVI lower than 100 mg/L and size 0.8 mm) at low OLR of 6 kg COD/m³.day after two weeks of operation.

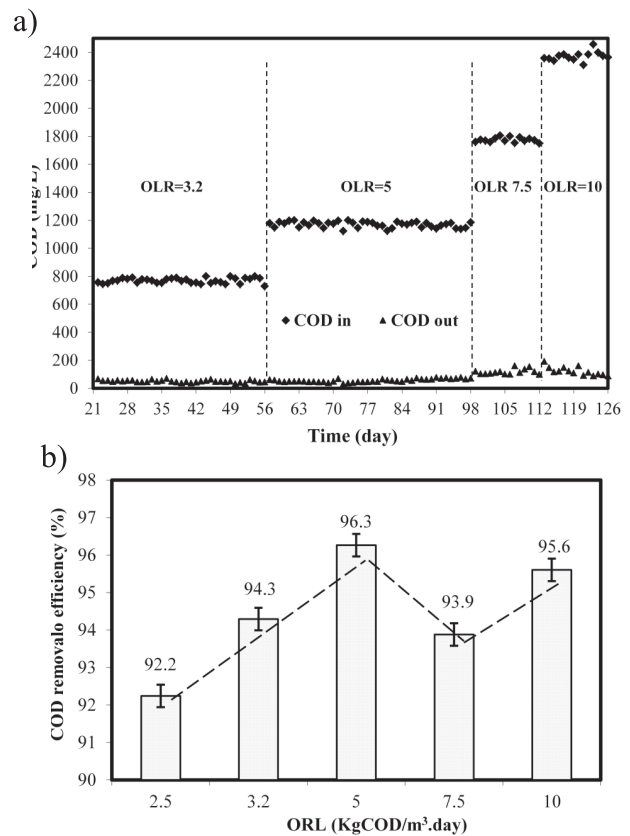


Fig. 8. a) COD concentrations in the influent and effluent and b) COD removal efficiency at different OLRs.

Substrate Removal Efficiency

After two weeks of operation, the rate of MLVSS/MLSS increased to the value of 79% and protozoa appeared abundantly in the sludge, while ciliates were predominant (Fig. 6). At the same time, chemical oxygen demand (COD) removal efficiency increased from 55.3% to a stable value of 90-91% at the end of the second week (Fig. 3a). In the third week, biomass in the reactor decreased further to 2,764 mg/L. However, COD removal efficiency was still higher than 92%, which is due to granules with high density of microorganisms increasing substance removal efficiency. From week 11 the development of granules according to the rise of OLR induced increases of MLVSS concentration and rate of MLVSS/MLSS up to more than 85% (Figs 7a and 7b), and COD removal efficiency rose to 93.9-96.3% simultaneously (Fig. 8b).

The results of monitoring the variation of COD and dissolve oxygen (DO) in a stable cycle at the OLR of 5 kg COD/m³.day showed that the substances were consumed quickly and fell to 60% in the first 20 minutes of aeration. Up to 40 minutes, more than 95% COD was removed. For the remaining aeration period, cell degradation occurred in the reactor and the extra cellular polymers were the adhesives that stuck the bacteria together. The elliptical-shaped airflow rolled the flocs into granules. The substance famine was identified by a sudden increase of DO while COD value decreased quickly from

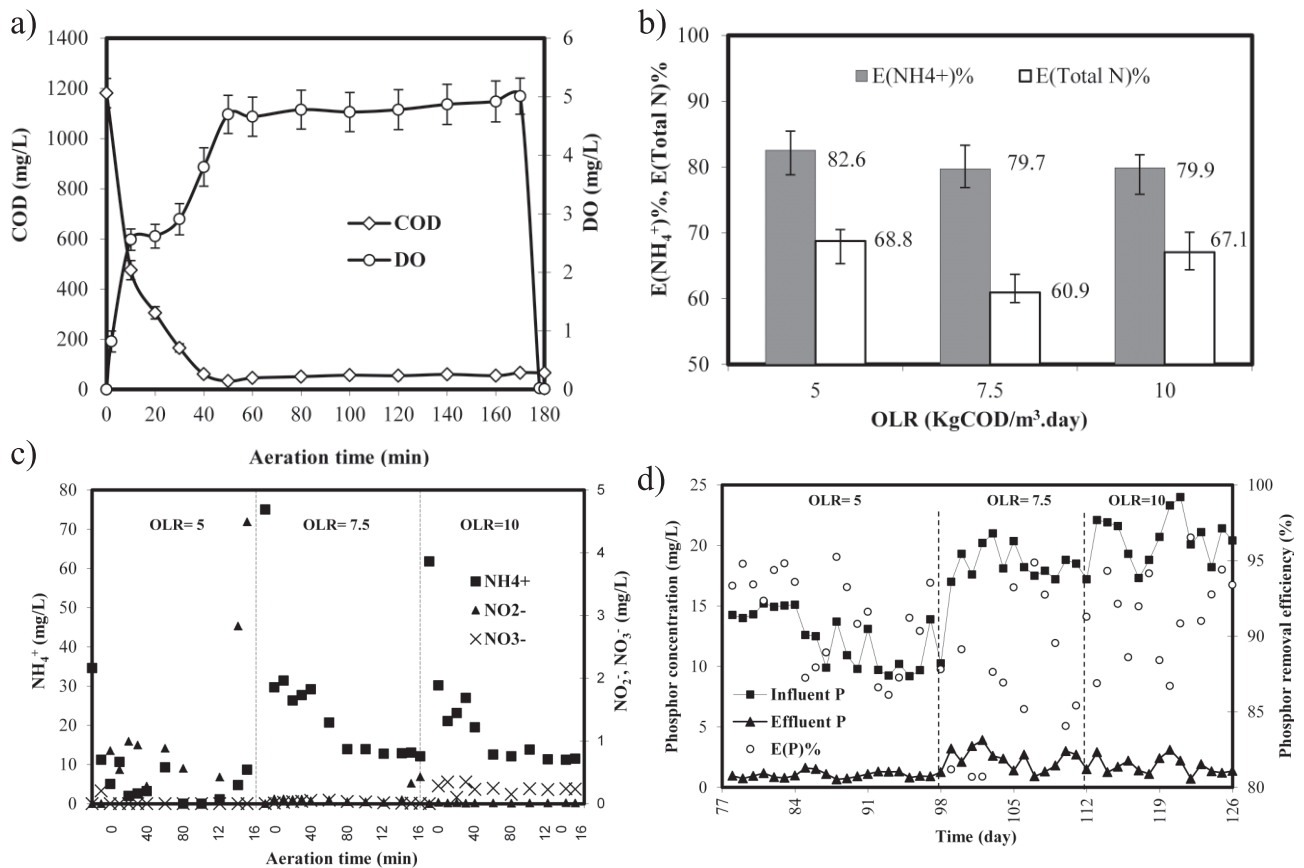


Fig. 9. Variations of: a) COD and DO to aeration time at the OLR of 5 kg COD/m³.day, b) NH₄⁺-N and total N removal efficiency to different OLRs, c) P removal efficiency to different OLRs, and d) NH₄⁺-N, NO₂⁻-N, and NO₃⁻-N to aeration time.

1,200 mg/L to less than 50 mg/L (Fig. 9a). When the substance concentration remained high, the DO value in the reactor was quite low due to substance transformation and degradation.

Moreover, nitrogen and phosphorus removal also occurred simultaneously in the reaction tank. A significant decrease in NH₄⁺-N and TP values served as evidence of high removal efficiency. These values were 79.7-82.6% and 80-95% for NH₄⁺-N and TP at different OLRs, respectively (Figs 9b, 9c, and 9d). Meanwhile, NO₂⁻-N and NO₃⁻-N after treatment did not increase significantly, which indicated that most organic N and NH₄⁺-N were converted into biomass. The remnant was transferred into nitrate, and nitrite and was evaporated as free-ammonia. Total nitrogen removal efficiency was low at 60.9-68.8% due to high DO, and low COD in the substance famine phase does not created favourable conditions for denitrification. Low nitrogen removal efficiency was also reported in [28]. At the operation mode without control of dissolve oxygen (DO), the maximum of total inorganic nitrogen (TIN) removal efficiencies reached 67.8-71.5%. Especially, Liu et al. [29, 30] got around 50% for total nitrogen removal efficiency in pilot-scale SBRs for real wastewater treatment. They concluded that to improve TIN removal efficiency, the operation mode should control DO or alternating anoxic/oxic combined with the

step-feeding mode, which supplies substrate for denitrification.

Conclusions

The formation of aerobic granules in SBAR using tapioca-processing wastewater as a substrate is visual evidence of successful aerobic granulation in a practical scenario. Aerobic granules quickly increased their density and diameter, which were maintained to a mean size of 2.5 mm at an OLR of 7.5 kgCOD/m³.day after 15 weeks' operation. Moreover, a high MLVSS/MLSS ratio of 80-90%, low SVI of 30-50 mL/g, the stronger and more compact structure of aerobic granular sludge, as well as high COD, NH₄⁺-N, and TP removal efficiency of 93.9-96.3%, 79.7-82.6%, and 80-95% respectively, confirmed the superiority of aerobic granules in SBAR over conventional activated sludge.

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